GENERAL NOTES:

- 1. ALL FOUNDATION WORK AND MATERIALS SHALL BE IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE AND ACI 318
- 2. PRIOR TO CONSTRUCTION ALL VEGETATION, STUMPS, ROOTS, FOREIGN MATERIAL AND SURFICIAL TOPSOIL SHALL BE REMOVED FROM THE AREA UNDER THE FOUNDATION AND TO A MINIMUM DISTANCE OF 5 FEET BEYOND THE LIMITS OF THE PROPOSED BUILDING. AFTER THIS STRIPING AND CLEARING HAS BEEN COMPLETED THE EXPOSED NATURAL SOILS SHALL BE COMPACTED TO 98% OF MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.
- GROUNDWATER LEVELS SHALL BE CONTROLLED TO A MINIMUM OF 2 FEET BELOW THE CONSTRUCTION LEVEL. GROUNDWATER ELEVATIONS MAY FLUCTUATE DURING CONSTRUCTION THEREFORE TEMPOARY DEWATERING MAY BE NESSASARY TO CONTROL THE GROUNDWATER LEVELS.
- 4. ALL FILL MATERIAL SHALL BE PLACED IN LIFTS NOT TO EXCEED 6 INCHES AND SHALL BE COMPACTED TO 98% OF MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.
- 5. IF "PUMPING" OF THE NEAR SURFACE SOILS OR FILL MATERIAL OCCURS DURING CONSTRUCTION WHICH RESULTS IN STRENGTH LOSS OF THE SUBSEQUENT SOIL, WORK SHALL BE TERMINATED IN THESE AREAS AND THE DISTURBED SOILS REMOVED. IN LEU OF REMOVING THE DISTURBED SOILS THE EXCESS MOISTURE MAY BE ALLOWED TO DISSAPATE AND THE SOIL RE-COMPACTED.

6. ALL FOOTINGS HAVE BEEN DESIGNED FOR THE FOLLOWING ASSUMED SOIL PROPERTIES: BEARING CAPACITY = 2.000 PSF

ANGLE OF INTERNAL FRICTION = 28 DEGREES COEFFICIENT OF FRICTION = 0.45 SOIL WEIGHT = 110 LBS PER CUBIC FOOT

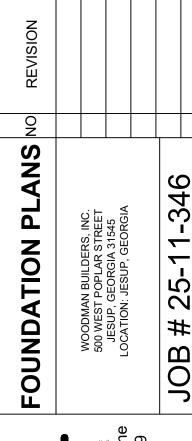
IF IT IS DETERMINED AFTER THE SOILS SURVEY THAT THE ACTUAL SOIL PROPERTIES ARE DIFFERENT THAN THESE ASSUMED VALUES, THE CONTRACTOR SHALL FOLLOW THE RECCOMENDATIONS OF THE GEOTECHNICAL ENGINEER.

STRUCTURAL DESIGN LOADS

BUILDING CODE: IBC 18

- 7. THE OUTLYING PERIMETER AREAS OF THE PROPOSED BUILDING SHALL BE GRADED IN SUCH A WAY AS TO PROVIDE POSITIVE DRAINAGE AWAY FROM THE PROPSED BUILDING.
- 8. A VAPOR RETARDER SHALLBE INSTALLED UNDERNEATH THE SLAB CONSISTING OF 6 MIL MINIMUM POLYETHYLENE WITH JOINTS LAPPED NOT LESS THAN 6 INCHES AND SEALED.
- 9. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS.
- 10. ALL CONCRETE SHALL CONTAIN 2.5% TO 6% ENTRAINED AIR TO ENHANCE FROST RESISTANCE.
- 11. THE MAXIMUM WATER CEMENT RATIO OF THE CONCRETE SHALL BE 0.55
- 12. THE SLUMP LIMITS OF ALL CONCRETE SHALL BE 2 4 INCHES.
- 13. ALL CONCRETE SHALL BE MIXED UNTIL THEIRE IS A UNIFORM DISTRIBUTION OF MATERIALS IN ACCORDANCE WITH ACI 318.
- 14. FOOTINGS SHALL BE CONSTRUCTED UTILIZING FORMS TO INSURE MINIMUM REBAR COVER IS MAINTAINED.
- 15. ALL REINFORCING BARS THAT DO NOT REQUIRE WELDING SHALL CONFORM TO ASTM-615. GRADE 60. ALL REINFORCING BARS THAT ARE TO BE WELDED SHALL CONFORM TO ASTM A706, GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM-185.
- 16. THE NUMBER 4 REBAR IN THE SLAB TURN DOWN SHALL BE CONTINUOUS FOR THE ENTIRE PERIMETER OF THE FOUNDATION AND SHALL BE LAPED SPLICED AT TERMINAL POINTS IN ORDER TO MAINTAIN
- 17. THE SLAB REINFORCING INCLUDING THE WELDED WIRE FABRIC AND HAIRPINS SHALLNOT BE CUT DURING OR ANYTIME AFTER CONSTRUCTION SINCE THIS REINFORCEMENT PROVIDES STRUCTURAL STABILITY FOR THE BUILDING.
- 18. SINCE PASSIVE RESISTANCE OF THE SOIL IS AN INTEGRAL PART OF THE ABILITY OF THE FOUNDATION TO RESIST HORIZONTAL FORCES THAT WILL BE PRESENT WHEN THE DESIGN LOADS ARE APPLIED TO THE BUILDING SYSTEM, FUTURE EXCAVATION SHOULD NOT TAKE PLACE WITHIN 50 FEET OF THE BUILDING.
- 19. CONTROL JOINTS SHALL BE INSTALLED IN THE FOUNDATION AT INTERVALS NOT TO EXCEED 15 FEET.
- 20. MAINTAIN 3 INCHES MINIMUM CLEARANCE FOR ALL REBAR AND ANCHOR BOLTS UNLESS OTHERWISE NOTED.

WIDTH (FT)	= 48.0/20.0	RISK CATEGORY	= II-NORMAL
LENGTH (FT)	= 100.0/30.0	SEISMIC SITE CLASS	= D
EAVE HEÌGHT (FT)	= 12.0/12.0/14.0/14.0	SEISMIC COEFFICIENT	= 0.28
ROOF SLOPE (RISE/12)	= 4.0/3.0	MAPPED RESPONSE (Ss)	= 0.1731
,		MAPPED RESPONSE (S1)	= 0.0777
DEAD LOAD (PSF)	= 2.19/2.91	DESIGN CATEGORY (SDC)	= B
COLLATERAL LOÁD (PSF)	= 5.0/1.0	IMPORTANCE - SEISMIC	= 1.00
ROOF LIVE LOAD (PSF)	= 20.0	SITE COEFFICIENT (Fa)	= 1.6
LIVE LOAD REDUCTION	= YES	SITE COEFFICIENT (FV)	= 2.4
		DESIGN RESPONSE (Sms)	= 0.2770
GROUND SNOW LOAD (PSF)	= 0.0	DESIGN RESPONSE (Sm1)	= 0.1865
ROOF SNOW LOAD (PSF)	= 0.0	DESIGN RESPONSE (Sds)	= 0.1846
THERMAL COEFFICIENT (Ct)	= 1.0	DESIGN RESPONSE (Sd1)	
IMPOATANCE - SNOW	= 1.0	RES MOD FACTOR (Mom) R	= 3.0
SNOW EXPOSURE (Ce)	= 1.0	APP PERIOD (MOMENT) Ta	= 0.2573/2476
		RES MOD FACTOR (Brc) R	= 3.0
ULTIMATE WIND (ULT) (MPH)	= 130.0	APP PERIOD (BRACED) Ta	= 0.1600/0.1543
NOMINAL WIND (ASD) (MPH)	= 100.7	TRANSVERSE SYSTEM	= MOMENT FRAMES
RISK CATEGORY	= II-NORMAL	LONGITUDINAL SYSTEM (ROOF)	= BRACED FRAMES
WIND EXPOSURE	= B	LONGITUDINAL SYSTEM (FSW)	= MOMENT FRAMES/CANT. COLUMN
CLOSED/OPEN/PARTIAL	= CLOSED/OPEN	LONGITUDINAL SYSTEM (BSW)	= MOMENT FRAMES/CANT. COLUMN
INTERNAL GCpi	= 0.18/-0.18/0.00/0.00		

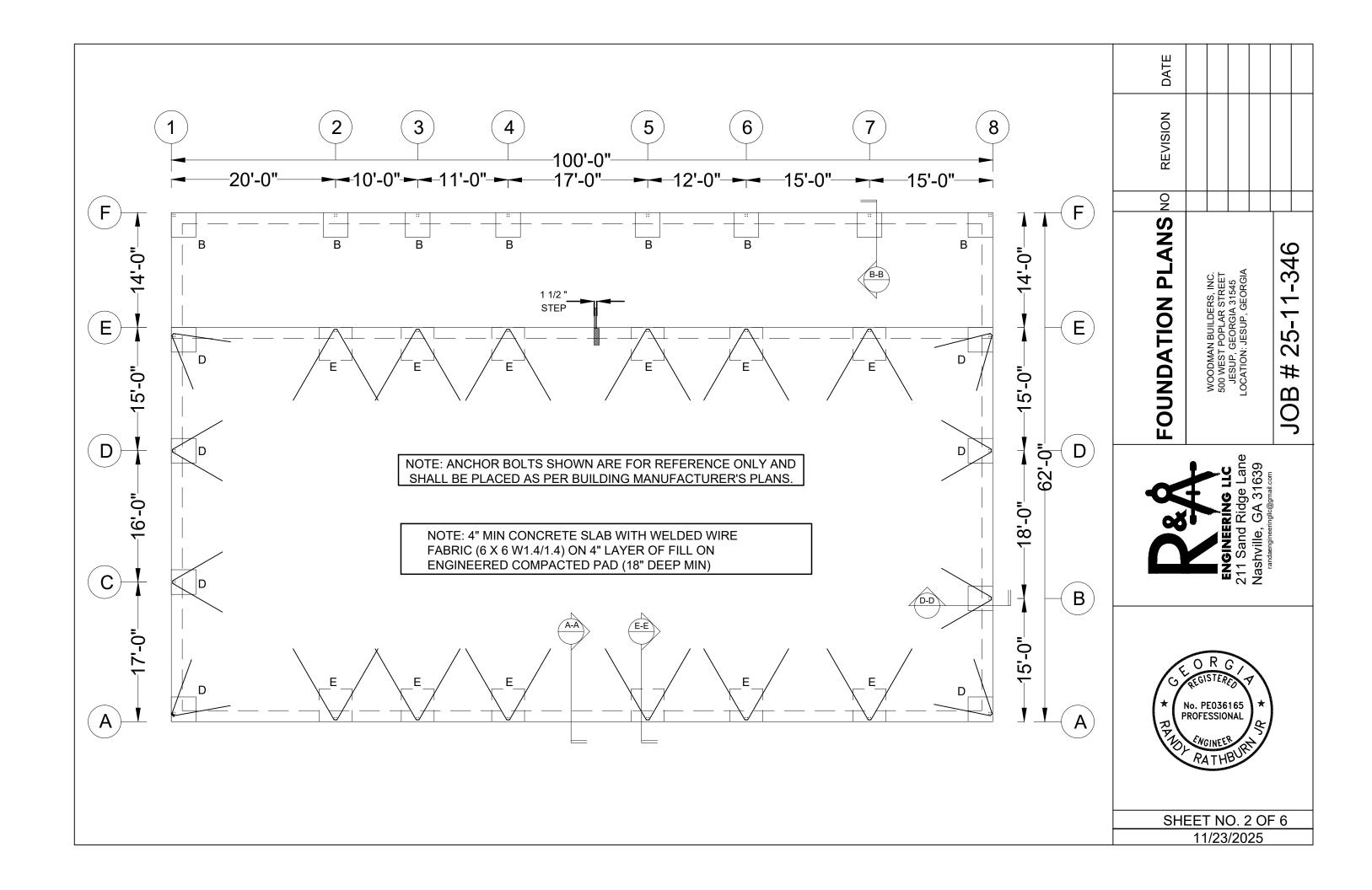


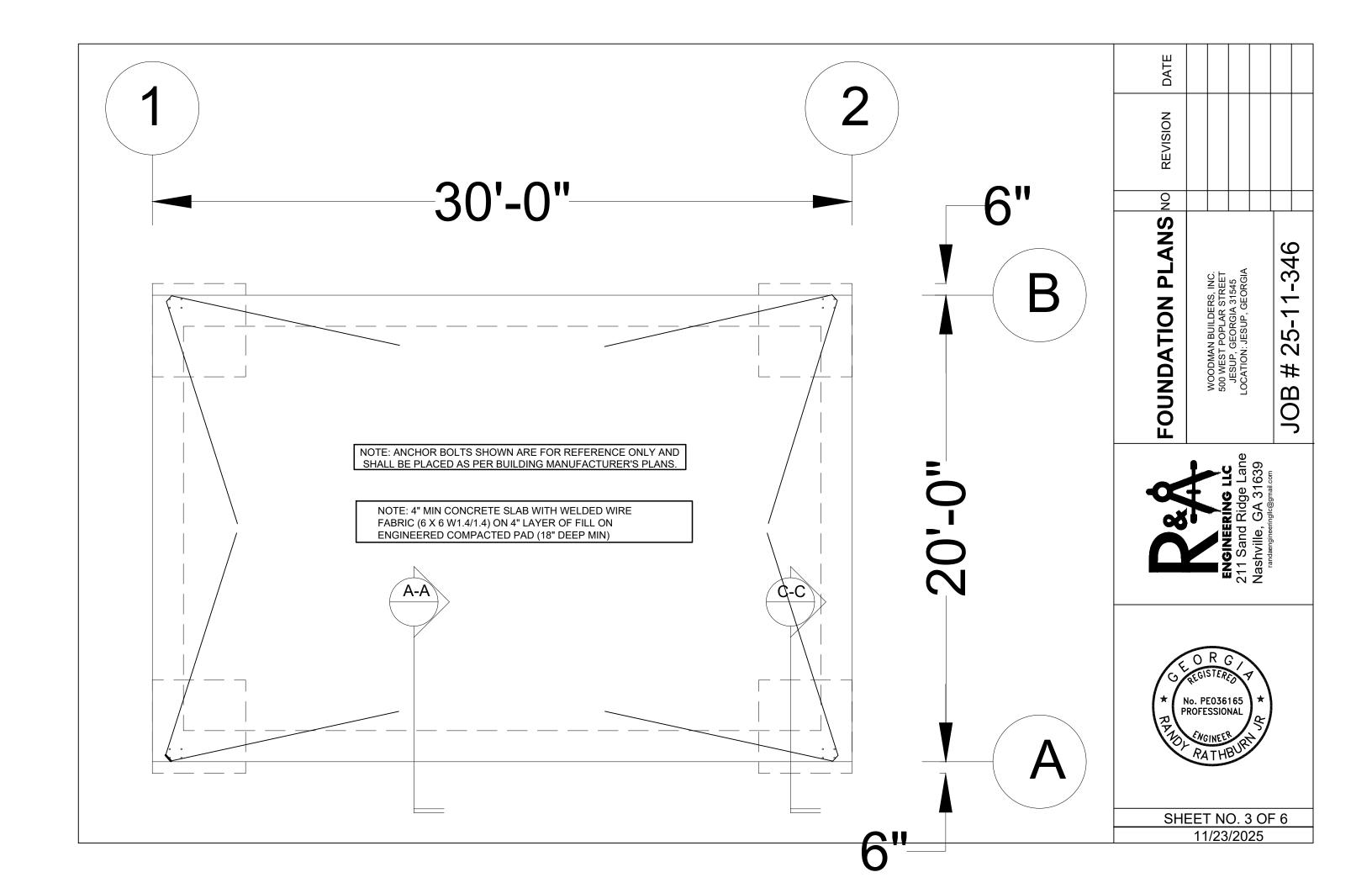


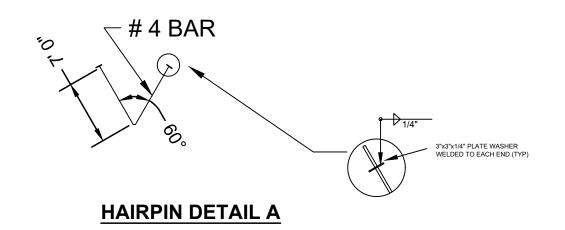


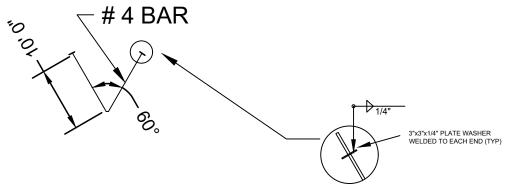
SHEET NO. 1 OF 6 11/23/2025

^{*} THE SEISMIC ANALYSIS PROCEDURE USED ON THIS STRUCTURE IS THE EQUILIVIENT LATERAL FORCE PROCEDURE

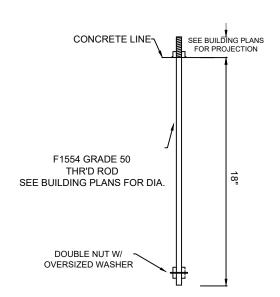




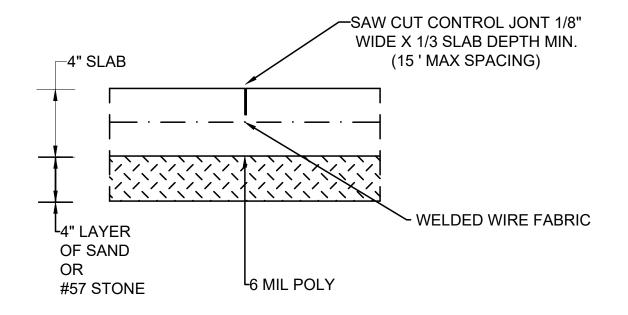


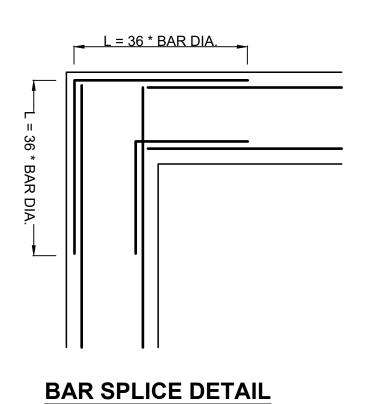


HAIRPIN DETAIL B



ANCHOR BOLT DETAIL





	FOUNDATION PLANS NO	O _N	REVISION	DATE	
•					
(WOODMAN BUILDERS, INC. 500 WEST POPLAR STREET				
ane	JESUP, GEORGIA 31545 LOCATION: JESUP, GEORGIA				
39					
	10 # 26 11 3/R				
	000 # 50-11-040				





SHEET NO. 4 OF 6 11/23/2025

FOOTING	SIZE (LXWXD)	REBAR REQ.	HAIRPIN REQ
B-B	3' X 3' X 2'	(4) #4 BARS TOP & BOTTOM EACH WAY EQ SP	NONE
C-C	4.5' X 4.5' X 2.5'	(6) #5 BARS TOP & BOTTOM EACH WAY EQ SP	SEE HAIRPIN DETAIL B
D-D	3' X 3' X 2'	(4) #4 BARS TOP & BOTTOM EACH WAY EQ SP	SEE HAIRPIN DETAIL A
E-E	4' X 4' X 2.5'	(6) #5 BARS TOP & BOTTOM EACH WAY EQ SP	SEE HAIRPIN DETAIL B

FOOTING SCHEDULE

(S) (*(足)	DeA	FOUNDATION PLANS NO	OZ	REVISION	DATE
No. F	*				
E036		WOODMAN BUILDERS, INC. 500 WEST POPLAR STREET			
G S165 DNAL	P P	JESUP, GEORGIA 31545 LOCATION: JESUP, GEORGIA			
× × ×	Nashville, GA 31639				
		10 p + 26 11 216			
		0+0-11-07 # dOc			



SHEET NO. 5 OF 6 11/23/2025

